HAMPDEN COUNTY WESTFIELD, MASSACHUSETTS

STEVENS PAPER COMPANY (UPPER) DAM MA-00075

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NATIONAL DAM INSPECTION PROGRAM CORPS OF ENGINEERS

U.S. Army Corps of Engineers New England District

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I. REPORT NUMBER	2. GOVT ACCESSION NO	3. RECIPIENT'S CATALOG NUMBER
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4. TITLE (and Substile)		S. TYPE OF REPORT & PERIOD COVERED
Stevens Paper Company (Upper) Dan	TI.	INSPECTION REPORT
NATIONAL PROGRAM FOR INSPECTION OF	NON-FEDERAL	6. PERFORMING ORG, REPORT NUMBER
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PERFORMING ORGANIZATION NAME AND ADDRES	. ·	10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
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IS. SUPPLEMENTARY NOTES

Cover program reads: Phase I Inspection Report, National Dam Inspection Program; however, the official title of the program is: National Program for Inspection of Non-Federal Dams; use cover date for date of report.

19. KEY WORDS (Continue on severee side if necessary and identify by block number)

DAMS, INSPECTION, DAM SAFETY,

Hampden County Westfield, Massachusetts

20. ABSTRACT (Continue on reverse side if necessary and identify by black number)

The Stevens Paper Co. (Upper) Dam - MA 00075 has been found to have a "low" hazard potential. Based on sketches contained in the state inspection reports, the dam consists of a stone masonry spillway 150 feet long and a maximum height of 18 feet near the right abutment. The average height of the spillway is approximately 15 feet.





One Center Plaza Boston, Massachusetts 02108 617 742-5151

June 21, 1979

New England Division U.S. Army Corps of Engineers 424 Trapelo Road Waltham, MA 02154

Attention: Mr. E.P. Gould, Project Manager

National Dam Inspection Program Contract No. DACW 33-79-C-0053

Gentlemen:

As a result of a site visit conducted on June 18, 1979, the Stevens Paper Co. (Upper) Dam - MA 00075 has been found to have a "low" hazard potential. This brief letter report documenting the determination of a "low" hazard potential classification of the dam is submitted in lieu of a complete Phase I Investigation Report.

Based on sketches contained in the state inspection reports, included herein, Stevens Paper Co. (Upper) Dam consists of a stone masonry spillway 150 feet long and a maximum height of 18 feet near the right abutment. The average height of the spillway is approximately 15 feet. The abutment walls and embankments rise 8 feet above the spillway crest on both banks. The dam, which has a 1901 date on its southwesterly abutment, impounds the water of the Little River in the City of Westfield, Mass. The dam was originally constructed to provide water power to the adjacent mill. The mill was not in operation at the time of the site visit.

Based on Corps of Engineers Guidelines for Estimating Dam Failure Hydrographs, and assuming that a failure would occur along 40 percent of the length of the dam structure, the peak dry weather flow (pool at spillway crest) is estimated to be 5,860 cfs and the peak wet weather flow (pool at top of dam) is estimated to be 19,280 cfs. The dry weather failure flow is less than the 10-year frequency flood presented in the Flood Insurance Study (FIS) for the City of Westfield, and no significant hazard is associated with such an event. The estimated wet weather failure flow is about equal to the 100-year flood of 18,300 cfs. The base flood which would result in a pool elevation at top of dam is equal to the 50-year flood. The hazard classification, therefore, depends on the increase in hazard, or lack thereof, between discharges equivalent to the 50-year and 100-year floods peaks.

New England Division Page 2 June 21, 1979

According to the FIS Flood Boundary Maps, no developed areas are affected by the 100-year flood between the Upper and Lower Stevens Co. Dams, a reach length of 1.3 miles. Crane Pond is formed by the Lower Stevens Co. Dam and would experience a surge as a result of a failure of the Upper Stevens Dam. However, assuming no reduction in the failure outflow in the 1.3 mile reach, a 5 foot freeboard would remain at the lower dam. Downstream of the Lower Stevens Dam is an extensive, underdeveloped flood plain which would attenuate any surge which might pass through Crane Pond.

The foregoing analysis considered a dam failure flow based on the downstream hydraulic height of the dam (downstream toe of dam to top of dam abutments). It should be noted that the enclosed profile presented in the FIS indicates that the upstream pool above the dam is almost totally silted in. This condition is exhibited in photographs 4 and 6. Consequently, a dam failure flow of 19,280 cfs is extremely conservative as there is essentially no dead storage in the upstream pool.

As there is no apparent increase in hazard above that which would exist prior to failure, the hazard potential of the Upper Stevens Dam is considered low. Based on this finding, the Phase I assessment of the condition of the dam has not been performed.

Very truly yours,

CAMP DRESSER & McKEE INC.

Roger H. Wood, Vice President

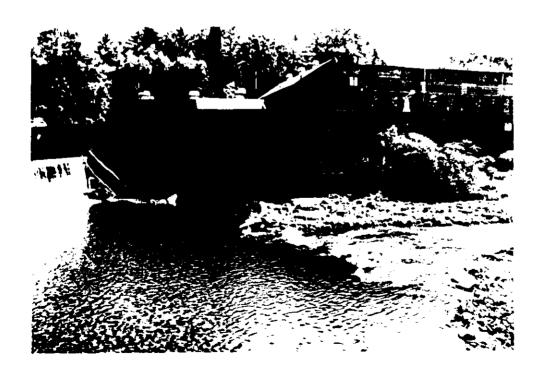
Roger N. Wood

RHW/je

Enclosures



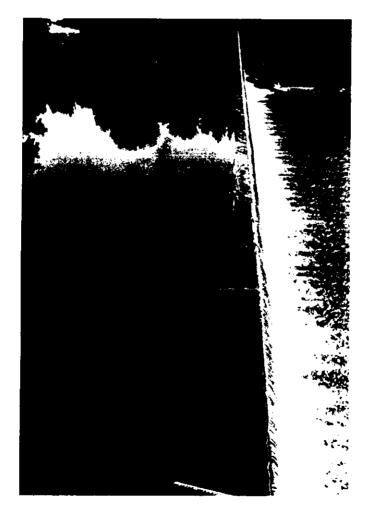
1. Overview of dam from downstream right abutment. Portion of Hortons Bridge located upstream of dam is shown in background.



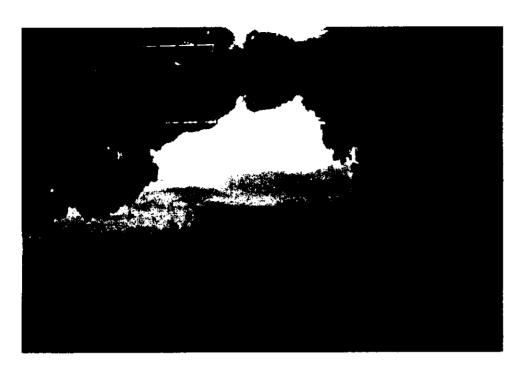
2. View of mill on downstream left bank.



3. Downstream channel from right abutment.



4. View of spillway crest from right abutment. Note shallow depth of upstream pool.



5. View of Little River downstream of Hortons Bridge. The dam and adjacent mill are shown at top of photograph.



6. View of Little River looking upstream from Hortons Bridge. Note shallow depth of upstream pool.



7. View of Crane Pond from Lower Stevens Dam left abutment.



8. View of Lower Stevens Dam and adjacent mill from Southwick Road Bridge.

PAGE _ lot = PROJECT Stovens (upas Dans DATE 6-15-79 _ DATE CHECKED. DETAIL Down Silves

150' wide stone masonry disti across Little River Height varies from 12 ft on east bank to 18 ft on west bank at downstream page.

Abotment height above spillury crest is 8 ff.

Florations
Spillury crost: 181.8
Hertment clav.: 189.8
Too of spillury: Veries from 157.8 to 163.8

DRY WEATHER FAILURE is: water at spillway crest $4p_1 = 8/27 \text{ (16)} + \sqrt{9} (76)^{3/2}$; $4p_2 = 40.7 \times 150' = 60'$ $4p_3 = 8/27 \times 60 \times (32.2)^{.5} (15)^{1.5} = 5,860 \text{ cris}$

from Flood Insurance Study (FIS) for the City of Westfield, the 10-YR tresvensy flood on the Zittle River at Upper Stowns Dani 13 6, 400 cts ... 5,960 cts world not present very significant hours.

WET WENTHER FAILURE ie: water at top of sam

1.5

Flaw over spillway prior to tailure = 4.0 × 150 × (8) = 15,580 st.

which is equal to the 50-ye flood in FIS

Failure flow = 6/27 × 60 × (37.2)(23) + 13,580 × 60% = 11,130 + 8,150 = 19,200

Incresse due to tuilure = 19,280 -13,580 = 5,700 cts

100-YR flood flow = 18,300 cm, from FIS, which almost equals Failure flow of 19,280cm

From FIS Flood Boundary Maps, no developed access
are affected by the 100-TR Hood between the
Upper and Lower Steven's Units, a reach length
of, 1.3 miles.

Field inspection of the Lower Steven's Dun found an abotherist height of 15 ft. Estaven Spilland arest and top of duri.

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DEIAIL_			CHECKED D1	COM. 0122 0

The FIS indicates the head on the Lawer Dam at 100TR flow (18,00 cfs & dam) is ~9 ft. and at 500TR flow (35,00 cfs & dam) is ~13 ft.

There for the day failure surcharge of 5,700 fs from 13,580 cts to 19,280 cts will not cause the day to be overtopped.

Inspection of the 100-TR flood plain between the Lower Stevens Darn and the Penn Central R.R. crossing, approx. 1.5 miles downstream, indicates more than adoquate surcharge-storage to attenuate the dark tailure surge that would pass the Lower Darn. The industrial withings which are lower Darn. The industrial withings which are lowered along Ponders Hollaw Kd. withing the 100-yr. That plain asjuciant to the Renn Guitral R.K. would not be significantly flooded over and above about about the law of the content of the prior to the count tailure.

It is noted that the foresping analysis asomes that the failure or upper Steven's Donn would rivolve the foll knowning the structure (% = 15'). However field surveyed cross sections obtained during the FIS and verified by tield inspection show that upstream pool is completely sitted in with a maximum where depths below spillway crest of 6 th. Therefore the above din failure flow is considered conservative.

It is therefore consoluted that a wet wather failure of the lope Jeven's Dans would not increase the downstrain factorist beyond that which existed immeadiately prior to the failure.

3.2 Hydrologic Analyses (continued)

Table 1b

<u>Discharge-Frequency Relationships</u>

<u>Little River at Westfield, Mass.</u>

Location	Drainage Arca		Disch	arge-cfs*	
	Square Miles	10 yrs	<u>50 yrs</u>	<u>100 yrs</u>	<u>500 yrs</u>
Above mouth	84.0	6,800	14,600	19,300	35,900
Above lower dam	81.0	6,600	14,200	18,900	35,000
Above upper dam	77.7	6,400	13,800	18,300	34,000
Above Munn Brook	56.5	5,100	11,100	14,700	27,200

E Discharge-frequency relationships for the remaining watercourses which were studied in the city of Westfield were developed by regional-frequency analysis (Reference 11) and routed by standard methods to the study areas. Adopted flows for the watercourses are listed in Table 1c.

Table 1c

<u>Discharge-Frequency Relationships</u>

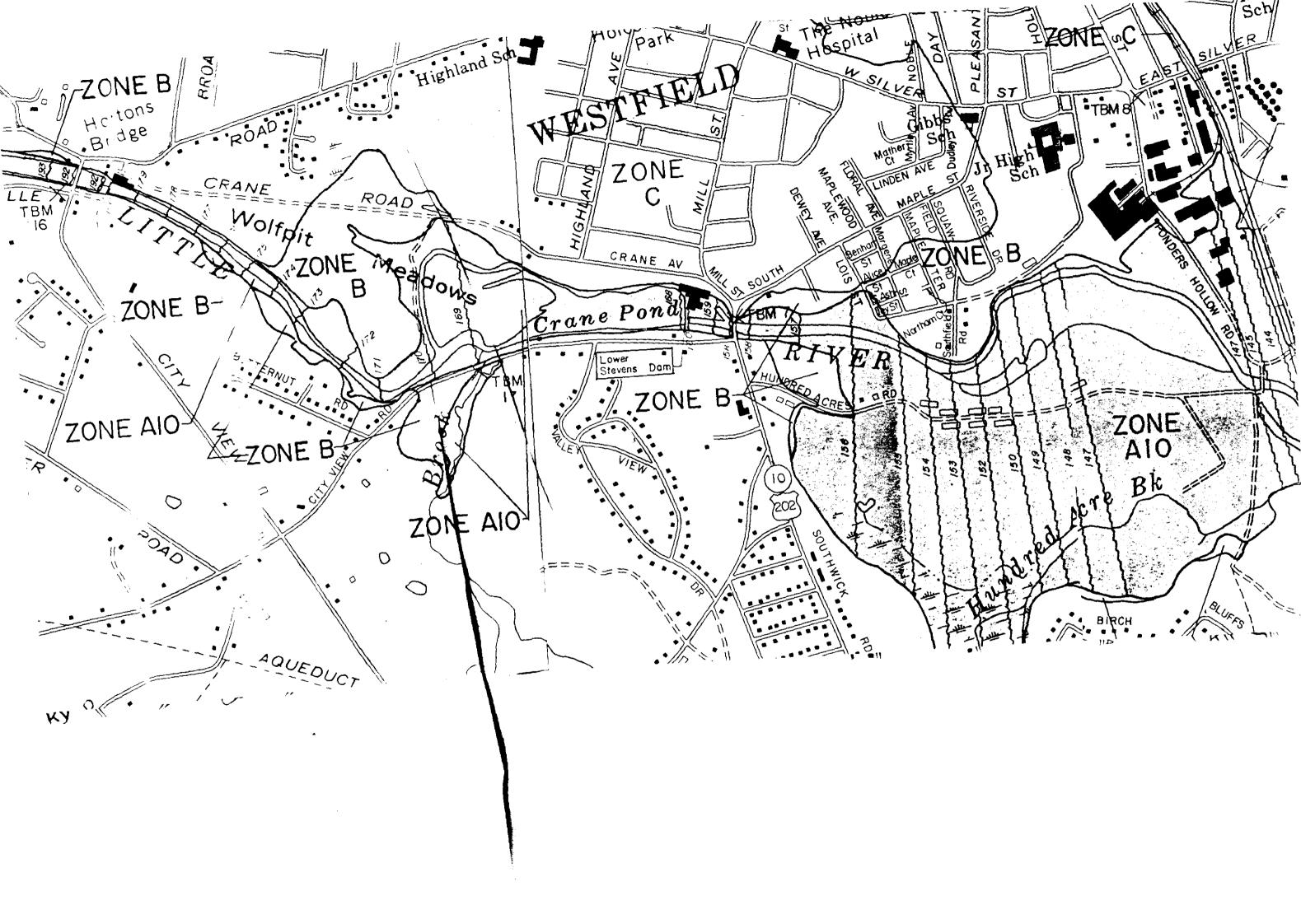
<u>Great, Powdermill, and Munn Brooks, Westfield, Mass.</u>

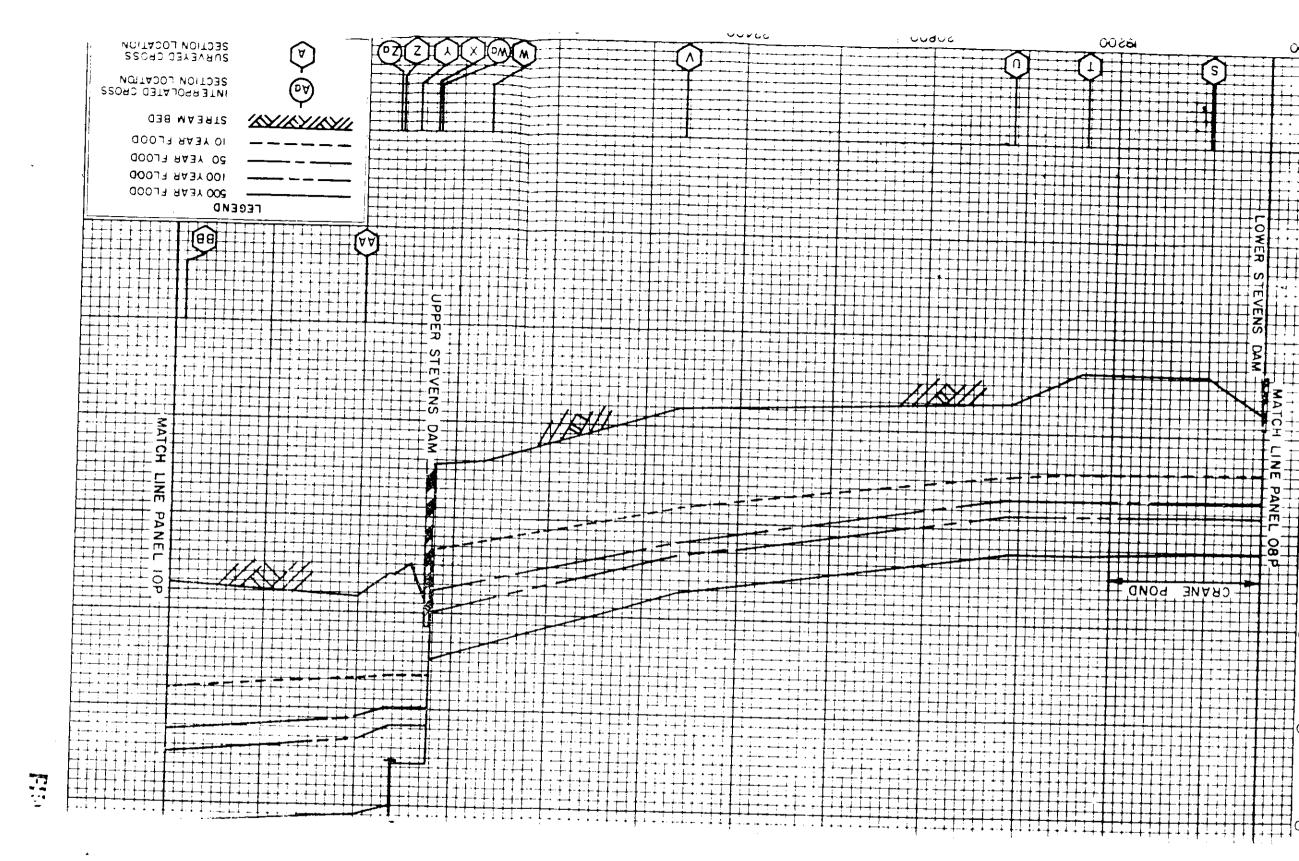
Location	Drainage Area		Discha	rge-cfs*	
	Square Miles	<u>10 yrs</u>	<u>50 yrs</u>	<u>100 yrs</u>	500 yrs
Great Brook	24.7	600	900	1,000	1,300
Powdermill Brook	19.1	75	750	1,000	1,400
Munn Brook	21.7	1,300	2,200	2,600	3,700

3.3 Hydraulic Analyses

- A Hydraulic characteristics of watercourses in the city of Westfield were analyzed, to estimate flood elevations of the selected recurrence intervals along each watercourse studied in detail.
- B Water-surface elevations for floods of the selected recurrence intervals were computed using the U.S. Army Corps of Engineers HEC-2 step-backwater computer program (References 12, 13, and 14). Cross sections for backwater analyses of watercourses studied in detail were field surveyed at specific locations

^{*}Cubic Feet Per Second.





(1)	LOCATION:
	City/maxx Westfield . County Hampden . Dam No. 2-7-329-6
٠.	Name of Dam Stevens Paper Company - Upper Dam Mass. Rect. Topo Sheet No. 9D . Coordinates: N 410,600 , E 251,000
٠.	Inspected by: Russell C. Salls, P.E. On Dec. 10, 1973. Last Inspection 1970 See also Dam Number 2-7-329-5
(2.)	ORNER/S: As of <u>December, 1973</u> per: Assessors , Reg. of Deeds , Prev. Insp. , Per. Contact x .
	1. Stevens Paper Mills, Inc., Windsor, Connecticut 06095 Name St. α No. City/Town State Tel. No.
	2. Name St. α No. City/Town State Tel. No.
- <u>3v</u>	Name St. α No. City/Torm State Tel. No.
\ ,	CARETALER: (if any) e.g. superintendent, plant manager, appointed by absentee owner, appointed by multi owners. Mr. Robert Ray, Plant Engineer
	C/o Stevens Paper Mills. Inc., 77 Mill Street, Westfield, Ma. 01085 562-2315 Name St. & No. City/Town State Tel. No.
(<u>4.)</u>	DATA: No. of Pictures Taken None Sketches See description of Dam. Plans, Where None found.
(5)	DEGREE OF HAZARD: (if dam should fail completely)*
	1. Minor x
	2. Moderate 4. Disastrous
	Comments: Large undeveloped flood plain below dam before river reaches Lower Dam Number 2-7-329-5. *This rating may change as land use changes (future development).

	weation and Type: drop varies 12: to 18:
	Flashboard stanchions bent - some missing -
	Controls Yes , TYPE: no flashboard.
	Automatic . Manual X . Operative Yes , No X .
	Comments: Exposed red stone ledge in area at base wall
No. 2	4 diameter * boiler plate drawdown 10 from east end 2 Location and Type: through base spillway wall.
	Slide gate, rack and pinion mechanism - Controls Yes, Type: remotely operated.
	Automatic . Manual X . Operative Yes . No X .
	Gate covered with silt, etc. by floods. Stem of remote operating
	Comments: mechanism bent out - bearing removed - inoperative. Just upstream dam easterly side river - intake chamber for
No. 3	3 Location and Type: former power penstock - now intake for water used by Mill.
	Controls Yes . Type: Control wheel on deck for gate - additional gates . in Mill.
	Automatic . Manual X . Operative Yes X , No .
	Mill Maintenance Supervisor Kellogg said gates in Mill
	Comments: operated recently.
Drawd Comme	down present Yes X , No . Operative Yes , No X .
	STREAM FACE: Slope <u>Unknown</u> , Depth Water at Dam 6' Stone rial: Turf Brush a Trees Rock fill X . Masonry X .Wood
Other	Back spillway wall under water assumed to be stone masonry
Condi	ition: 1. Good X 3. Major Repairs
	2. Minor Repairs 4. Urgent Repairs .
C	
Comme	ents: Under water - conditions assumed
DAM DOW	Cut stone on crest overhangs NSTREAM FACE: Slope 1 in 6 Batter 8" to foot. Stone
Mater	rial: Turf . Brush & Trees . Rock Fill . Masonry X . Wood .
Other	·
Condi	tion: 1. Good X 3. Major Repairs
	2. Minor Repairs 4. Urgent Repairs
Comme	Unable to inspect spillway wall and toe due to 4 to 6 inches of water floonts: over wall. Wall apparently founded on red ledge. Noted three small holds
	100' downstream-

Width Ft. Height Ft. Waterial
Condition: 1. Good . 3. Major Repairs .
2. Minor Repairs 4. Urgent Repairs .
Comments: Flood training wall on east side built in 1957 to prevent water
from flowing through Mill Yard and Mill.
MATER LEVEL AT TIME OF INSPECTION: 1/3 Ft. Above X . Below
Top Dam X F.L. Principal Spillway
Other
8! ± west bank Normal Freeboard 12: east bank xx
SUMMARY OF DEFICIENCIES NOTED: Growth (Trees and Brush) on Embankment None
Animal Eurrows and Washouts None
Slight wear of masonry of abutment at Damage to Slopes or Top of Dam crest of spillway.
Cracked or Damaged Masonry None seen
Evidence of Seepage None noted
Three very small "tea cup size" boils about 100 ft. Evidence of Piring downstream on easterly side of river.
Leaks None noted
Erosion None noted
Trash and/or Debris Impeding Flow Some logs on crest spillway
Drawdown gate inoperative and covered with silt, sand and gravel by floods.
Other

	Safe	
•	Minor repairs needed X	
٠.	Conditionally safe - major repairs needed	
	Unsafe	
	Reservoir impoundment no longer exists (explain)	

(13.) REMARKS AND RECOMMENDATIONS: (Fully Explain)

This is an old stone masonry spillway dam with a 1901 date on its southwesterly abutment. It originally provided water power to the adjacent Mill but now the Upper Mill is using the old penstock to draw water for processing and fire protection and the old turbine pit is closed off.

According to the Mill Maintenance Supervisor the control at the penstock inlet has not been operated for some time but control valves and gates inside the Mill are operating. The penstock entry chamber is covered with a plank deck with the hand wheel controlling the intake gate projecting above it. Close inspection of the intake was not practical. There is a steel bar trash rack on the river side of the entry chamber.

In 1957 a concrete flood training wall was built on top of the old stone abutment wall on the northeasterly end of the dam with a leg extending into the slope to present flooding of the Mill and Mill Yard. At this time the shaft for the remote control for the drawdown gate in a recess in the abutment wall was forced outward making it inoperative. The concrete wall and stone masonry abutment are in good condition.

The old four foot diameter drawdown conduit through the base of the stone masonry spillway wall about ten feet from the east end was visible on the downstream face. Silt and other debris covered the upstream end and the gate. The Plant Maintenance Supervisor said that the gate was originally operated by a rack and pinion arrangement located under the water at the gate. The pinion was turned by means of a hand wheel located on the abutment through a system of gears. This drawdown has not been opened for years and the operating mechanism is inoperative.

RCS/js/sd

The alignment and grade of spillway structure and the abutments were true with no visible sags or bulges and no loose or missing stones noted. It appears to be built on red sand stone ledge.

About 100 feet downstream of the dam on the easterly side of the river, three small boils about the size of a tea cup were noted. These were pointed out to the Maintenance Supervisor and the Assistant Factory Manager. It is suggested that a close watch of these boils be maintained.

RCS/sd

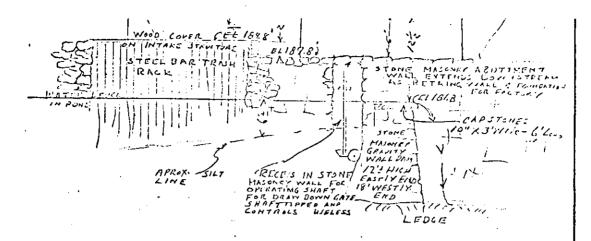
	Date December 10, 1973 City/Smoon Westfield
	Name of Dam <u>Stevens Paper Company -</u> Upper Dam
1.	liass. Rect. Location: Topo Sheet No. 90 Coordinates N 410,600 E 251,000
	Provide $8\frac{1}{2}$ " x 11" in clear copy of topo map with location of Dam clearly indicated,
	On Little River at Hortons Bridge - Granville Road, one mile from Western
	Avenue, 300 ft. east of bridge.
2.	Vinknown - but 1901 Year built date on right abutment. Year/s of subsequent repairs 1956 - Flood training wall on south side.
3.	Purpose of Dam: Water Supply For Mill Recreational X Flood Control Irrigation Other Formerly used for water power.
4.	Drainage Area: in excess = 48 sq. mi. acres.
	Type: City, Bus. & Ind. Dense Res. Suburban Rural, Farm 10%
	Wood & Scrub Land 90% Slope: Steep 20% Med. 60% Slight 20%
5.	Normal Ponding Area: 17 Acres; Ave. Depth 3 to 4
	Impoundment: 22.2 Million gals.; 68 acre ft. Silted in: Yes x No Approx. Amount Storage Area 40g
5.	No. and type of dwellings located adjacent to pond or reservoir
	i.e. summer homes etc. 8 to 12 full time residences plus Paper Mill.
7 •	Dimensions of Dam: Length 150 ft. Nax. Height 18 ft. Freeboard 125 ft. on west end.
	Slopes: Upstream Face <u>Unknown - under water.</u> Downstream Face <u>Appears to be 1 in 4 batter.</u>
	Width across top 6' - cut stone crest.

		Timber				<u> </u>	ther		
ΙΛ.	Dam	Type: Gravit Overfl		Straight Non-ov			Arched	·	_ Other
	Α.	Description of	present l	and usage	downst	ream of d	am:		
		80 % r	ural;	<u>20</u> %	urban	•	-	•	
	в.	Is there a stor could accommoda dam failure? Y	te the im	poundment	in the	event of			•
	C.	Character Downs	tream Val	ley: Narro	w	Wide	x	_ Devel	Loped
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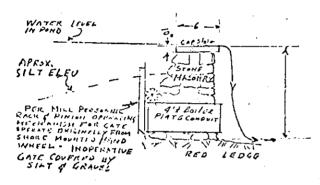
RCS/vk /sd Attachments Locus Plan Sketches

L

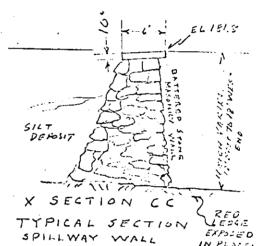
スペンス GEICK EVCLOST BAILDING ON STONE MASONEY - Mellivanori WOOD GOVER RED LEDGE DRAW DOWN COILS CAP - -IMPOUND MENT 150 LITTLE GREIT



X SECTION AA-SHOWING EAST ABUTMENT & INTAKE FOR MILL



X SECTION BB-THRU 4't DRAWDOWN CONDUIT - 10'+ FROM EAST END DAM



IN PLACES

